

# TWO-DIMENSIONAL FINITE ELEMENT ANALYSIS

- OTHER ELEMENT TYPES
- PLANAR ELASTICITY PROBLEM
- DISCUSSION ON ELEMENT CALCULATIONS
- BOUNDARY-CONDITION IMPOSITION
- AN EXAMPLE

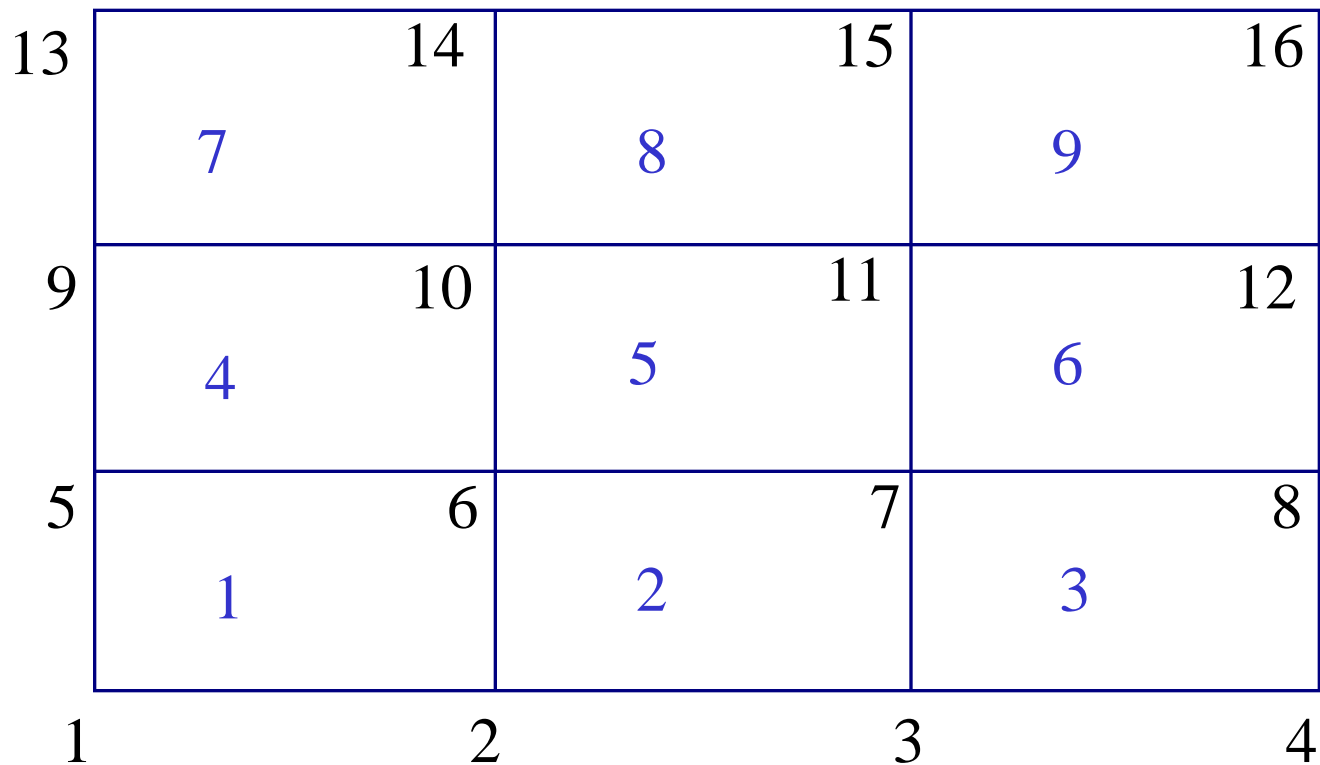
$$\int_B \left( \frac{\partial w}{\partial x} \frac{\partial v}{\partial x} + \frac{\partial w}{\partial y} \frac{\partial v}{\partial y} \right) dA = \int_B (rv) dA + \int_{\Gamma_D + \Gamma_N} \underbrace{\left( \frac{\partial w}{\partial x} n_x + \frac{\partial w}{\partial y} n_y \right)}_{\frac{\partial w}{\partial n}} v dS$$

THE WEAK FORMULATION

$$= \int_{\Gamma_N} \frac{\partial w}{\partial n} v dS$$

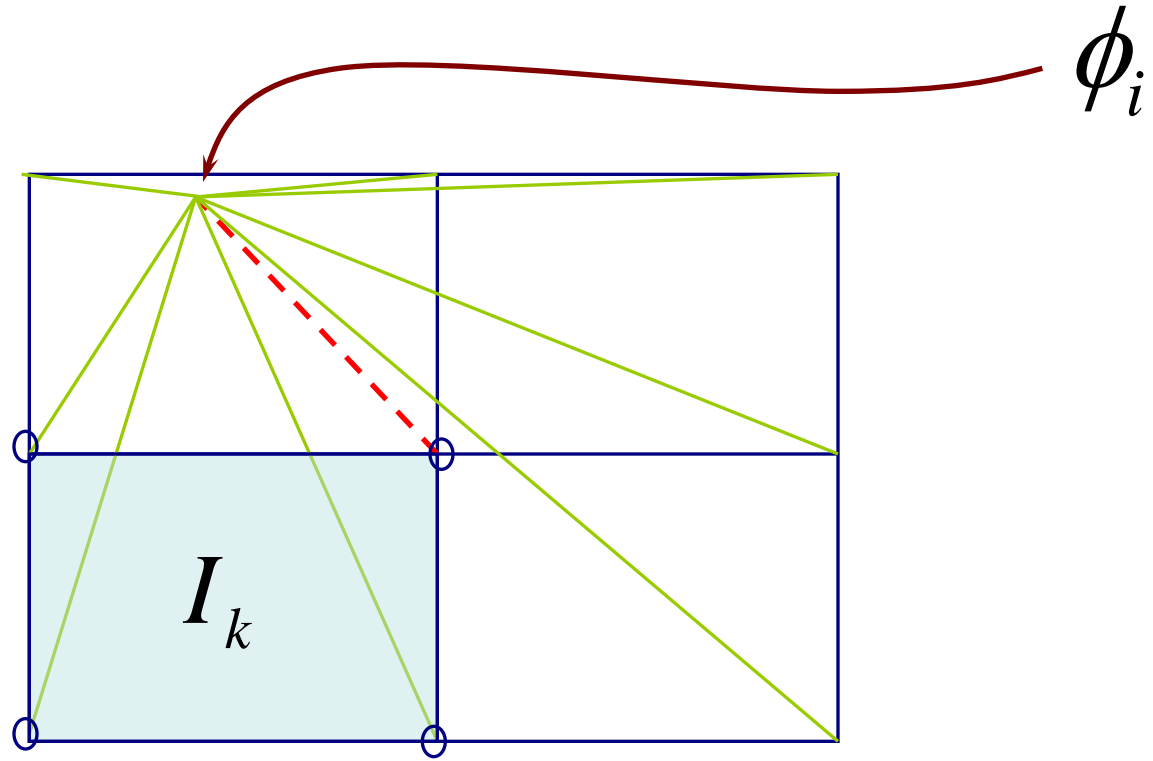
$$v = 0 \quad \text{on} \quad \Gamma_D$$

$$\frac{\partial w}{\partial n} \Big|_{\Gamma_N} = g(x(s), y(s))$$

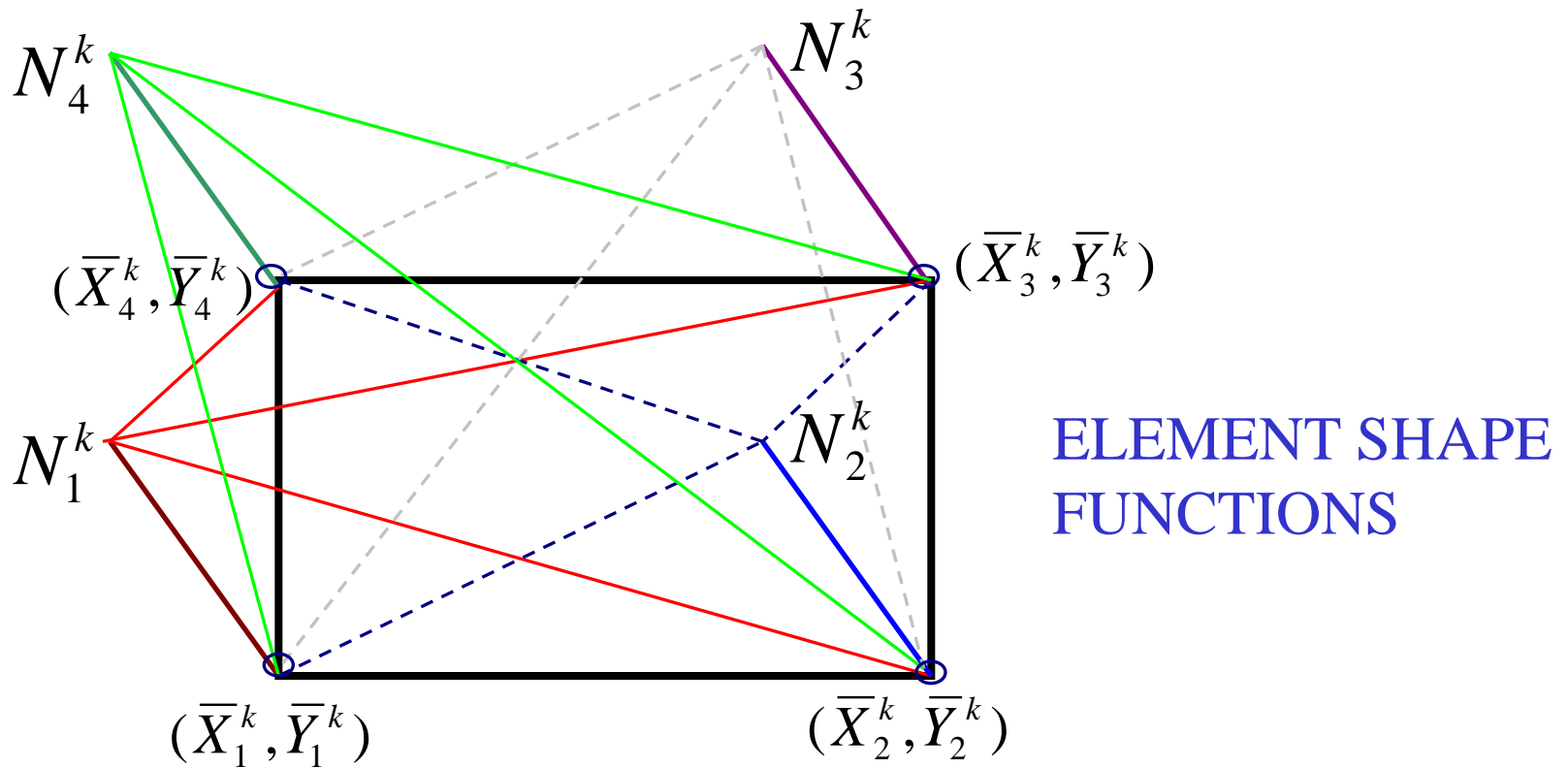


$$w_{FE}(x, y) = \sum_{i=1}^{16} \alpha_i \phi_i(x, y)$$

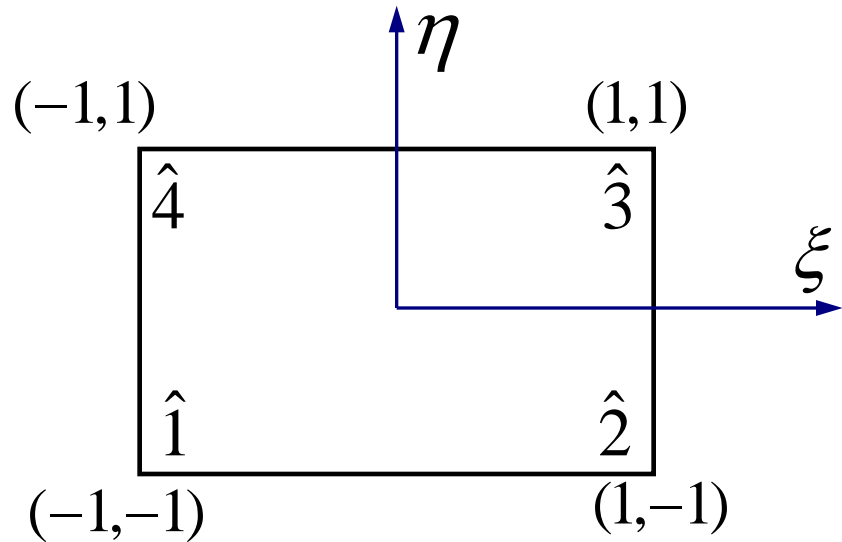
MESH OF RECTANGLES  
(QUADRILATERALS)



THE REGION WHERE BASIS FUNCTION  $\phi_i$  IS NON-ZERO



**MASTER ELEMENT**



WE ARE GOING TO THE MASTER ELEMENT RIGHT AWAY

$$x(\xi, \eta) = (a_1 + a_2\xi)(b_1 + b_2\eta)$$

**BILINEAR MAP**

$$\begin{array}{ll} x(-1, -1) = \bar{X}_1^k & x(1, -1) = \bar{X}_2^k \\ x(1, 1) = \bar{X}_3^k & x(-1, 1) = \bar{X}_4^k \end{array}$$

**SIMILARLY FOR  $y$**

**THIS GIVES**

$$x(\xi, \eta) = \bar{X}_1^k \frac{(1-\xi)(1-\eta)}{4} + \bar{X}_2^k \frac{(1+\xi)(1-\eta)}{4} + \bar{X}_3^k \frac{(1+\xi)(1+\eta)}{4} + \bar{X}_4^k \frac{(1-\xi)(1+\eta)}{4}$$

$$y(\xi, \eta) = \bar{Y}_1^k \frac{(1-\xi)(1-\eta)}{4} + \bar{Y}_2^k \frac{(1+\xi)(1-\eta)}{4} + \bar{Y}_3^k \frac{(1+\xi)(1+\eta)}{4} + \bar{Y}_4^k \frac{(1-\xi)(1+\eta)}{4}$$

IN THE MASTER ELEMENT, THE SHAPE FUNCTIONS BECOME

$$N_1^k(x, y) = \hat{N}_1(\xi, \eta) = \frac{(1-\xi)(1-\eta)}{4}$$

$$N_2^k(x, y) = \hat{N}_2(\xi, \eta) = \frac{(1+\xi)(1-\eta)}{4}$$

$$N_3^k(x, y) = \hat{N}_3(\xi, \eta) = \frac{(1+\xi)(1+\eta)}{4}$$

$$N_4^k(x, y) = \hat{N}_4(\xi, \eta) = \frac{(1-\xi)(1+\eta)}{4}$$

BILINEAR SHAPE  
FUNCTIONS



TENSOR PRODUCT  
FAMILY OF SHAPE  
FUNCTIONS and

SERENDIPITY  
FAMILY OF SHAPE  
FUNCTIONS

PRODUCT OF 1D-SHAPE FUNCTIONS  
IN THE TWO DIRECTIONS

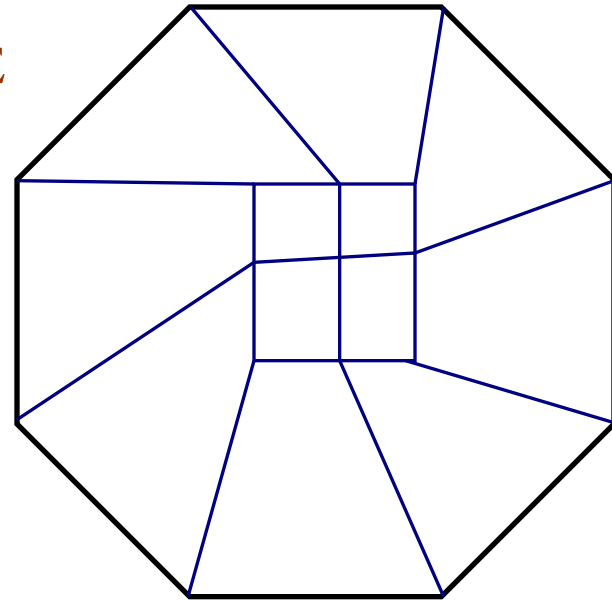
NOW THESE CAN BE USED FOR MESHES OF QUADRILATERALS ALSO

THE PROCEDURE FOR OBTAINING THE METRICS OF THE TRANSFORMATION REMAINS EXACTLY THE SAME

$$\frac{\partial x}{\partial \xi}, \frac{\partial x}{\partial \eta}, \frac{\partial y}{\partial \xi}, \frac{\partial y}{\partial \eta}$$



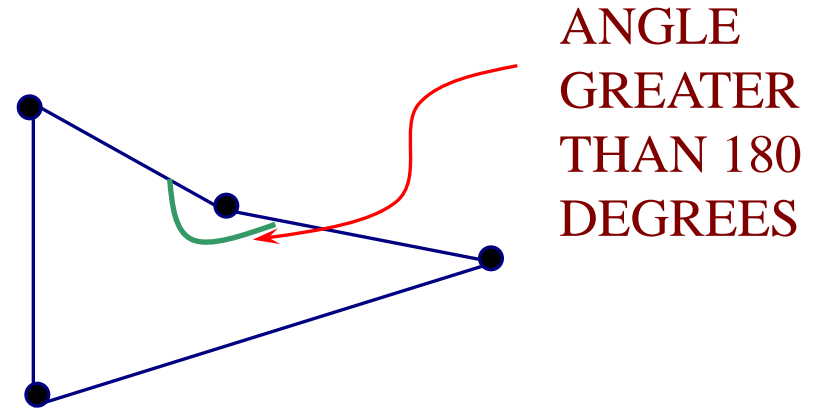
$$|J| \rightarrow \frac{\partial \xi}{\partial x}, \frac{\partial \xi}{\partial y}, \frac{\partial \eta}{\partial x}, \frac{\partial \eta}{\partial y}$$



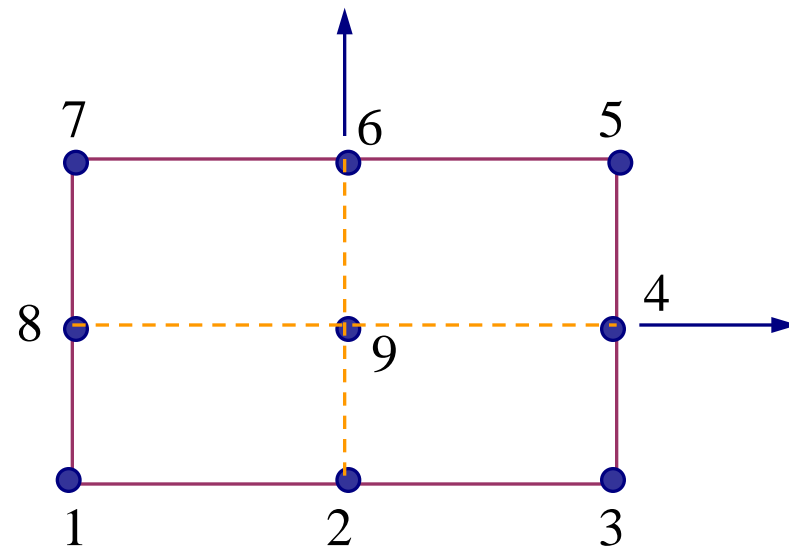
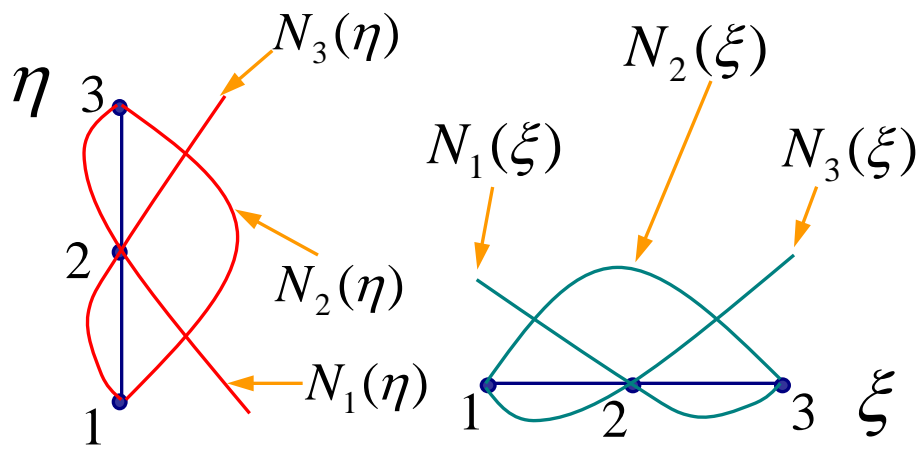
THE JACOBIAN IS NOW A FUNCTION OF  $\xi, \eta$  AND NOT A CONSTANT ANYMORE. HENCE, THE JACOBIAN HAS TO BE EVALUATED AT EACH INTEGRATION POINT



CERTAIN ELEMENT GEOMETRIES SHOULD BE AVOIDED (JACOBIAN BECOMES NEGATIVE OR ZERO). THESE ARE



**BI-QUADRATIC ELEMENTS ( 9 NODED )**



$$\hat{N}_1(\xi, \eta) = N_1(\xi)N_1(\eta)$$

$$\hat{N}_2(\xi, \eta) = N_2(\xi)N_1(\eta)$$

$$\hat{N}_3(\xi, \eta) = N_3(\xi)N_1(\eta)$$

$$\hat{N}_4(\xi, \eta) = N_3(\xi)N_2(\eta)$$

$$\hat{N}_5(\xi, \eta) = N_3(\xi)N_3(\eta)$$

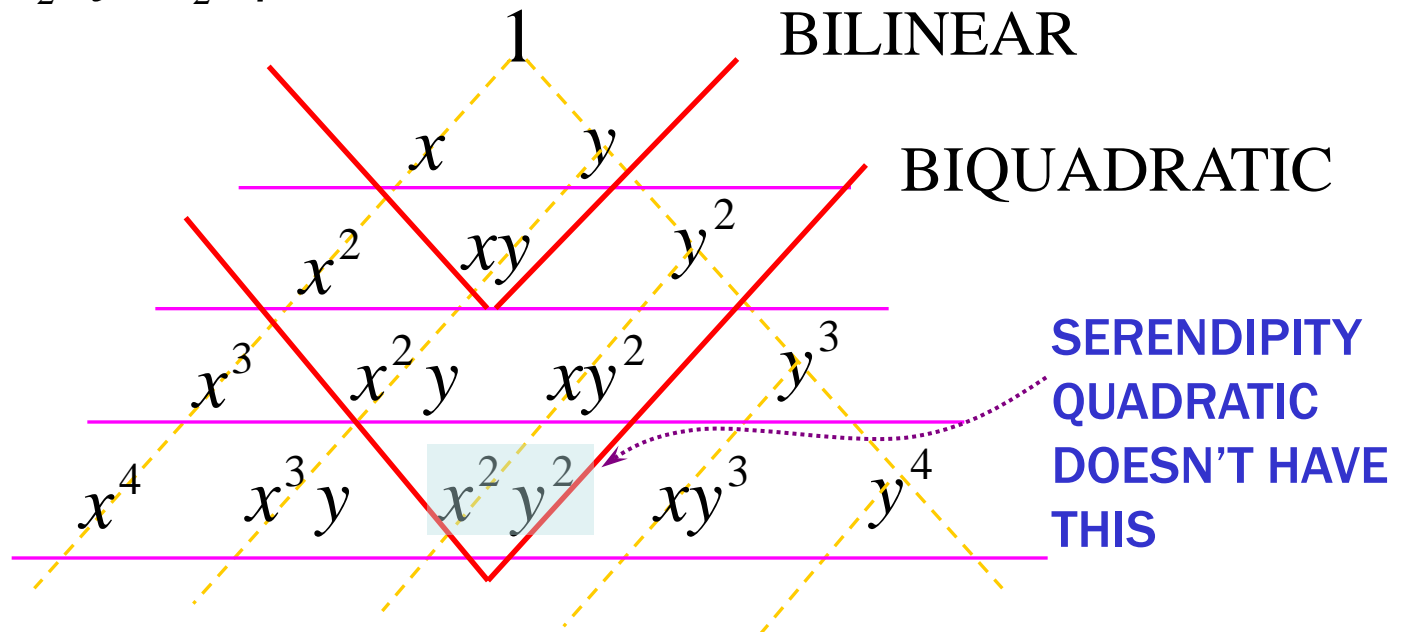
$$\hat{N}_6(\xi, \eta) = N_2(\xi)N_3(\eta)$$

$$\hat{N}_7(\xi, \eta) = N_1(\xi)N_3(\eta)$$

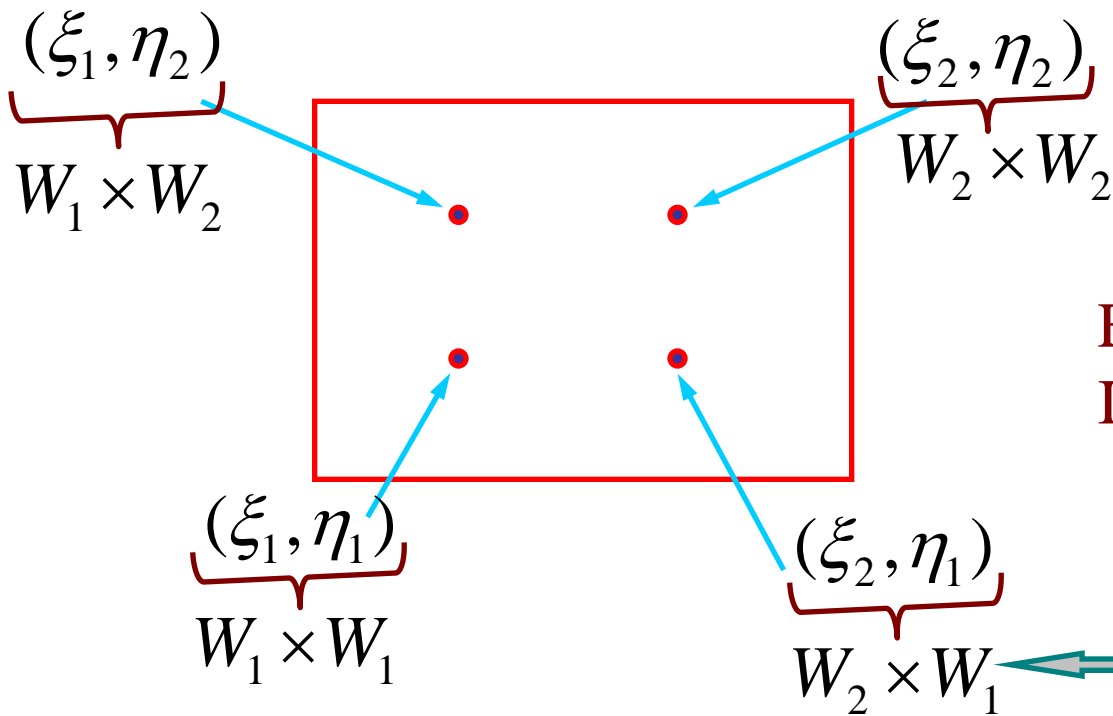
$$\hat{N}_8(\xi, \eta) = N_1(\xi)N_2(\eta)$$

$$\hat{N}_9(\xi, \eta) = N_2(\xi)N_2(\eta)$$

**MORE THAN COMPLETE!**

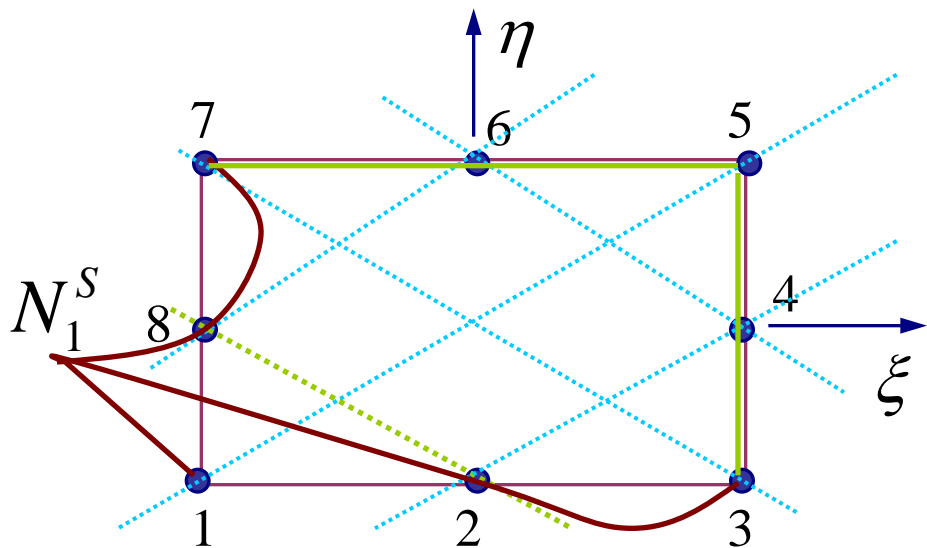


**CAN USE 1D INTEGRATION POINT IN BOTH DIRECTIONS**



EXAMPLE OF A 2 x 2  
INTEGRATION RULE

WEIGHTS ARE  
ALSO PRODUCTS  
OF 1D- WEIGHTS



$$N_1^S = \alpha(\xi + \eta + 1)(\xi - 1)(\eta - 1)$$

SERENDIPITY ELEMENTS ( 8 NODED)

# THE *i*th EQUATION

$$\int_B \left( \frac{\partial w_{FE}}{\partial x} \frac{\partial \phi_i}{\partial x} + \frac{\partial w_{FE}}{\partial y} \frac{\partial \phi_i}{\partial y} \right) dA = \int_B (r \phi_i) dA + \int_{\Gamma_N} \left( \frac{\partial w}{\partial n} \right) \phi_i dS$$



$$\sum_{k=1}^{NEL} \int_{I_k} \left( \frac{\partial w_{FE}}{\partial x} \frac{\partial \phi_i}{\partial x} + \frac{\partial w_{FE}}{\partial y} \frac{\partial \phi_i}{\partial y} \right) dA = \sum_{k=1}^{NEL} \left( \int_{I_k} (r \phi_i) dA + \int_{\partial I_k \cap \Gamma_N} \left( \frac{\partial w}{\partial n} \right) \phi_i dS \right)$$

INTEGRALS OVER ELEMENT AREA

ELEMENT EDGE INTEGRAL

$$\frac{\partial N_i^k}{\partial x} = \frac{\partial \hat{N}_i}{\partial \xi} \frac{\partial \xi}{\partial x} + \frac{\partial \hat{N}_i}{\partial \eta} \frac{\partial \eta}{\partial x}$$

$$\frac{\partial N_i^k}{\partial y} = \frac{\partial \hat{N}_i}{\partial \xi} \frac{\partial \xi}{\partial y} + \frac{\partial \hat{N}_i}{\partial \eta} \frac{\partial \eta}{\partial y}$$

CONVERSION OF  
DERIVATIVES TO  
MASTER ELEMENT

NEED TO GET  
THESE

$$\begin{Bmatrix} dx \\ dy \end{Bmatrix} = \begin{bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial x}{\partial \eta} \\ \frac{\partial y}{\partial \xi} & \frac{\partial y}{\partial \eta} \end{bmatrix} \begin{Bmatrix} d\xi \\ d\eta \end{Bmatrix}$$

$[J]$

JACOBIAN MATRIX

$$\begin{Bmatrix} d\xi \\ d\eta \end{Bmatrix} = \begin{bmatrix} \frac{\partial \xi}{\partial x} & \frac{\partial \xi}{\partial y} \\ \frac{\partial \eta}{\partial x} & \frac{\partial \eta}{\partial y} \end{bmatrix} \begin{Bmatrix} dx \\ dy \end{Bmatrix}$$

$[J]^{-1}$

$$\begin{array}{cc} \frac{\partial x}{\partial \xi} & \frac{\partial x}{\partial \eta} \\ \frac{\partial y}{\partial \xi} & \frac{\partial y}{\partial \eta} \end{array}$$

THIS CAN BE  
EVALUATED

$$|J| = \frac{\partial x}{\partial \xi} \frac{\partial y}{\partial \eta} - \frac{\partial x}{\partial \eta} \frac{\partial y}{\partial \xi}$$

THE JACOBIAN

$$\begin{array}{cc} \frac{\partial \xi}{\partial x} = \frac{1}{|J|} \frac{\partial y}{\partial \eta} & \frac{\partial \xi}{\partial y} = -\frac{1}{|J|} \frac{\partial x}{\partial \eta} \\ \frac{\partial \eta}{\partial x} = -\frac{1}{|J|} \frac{\partial y}{\partial \xi} & \frac{\partial \eta}{\partial y} = \frac{1}{|J|} \frac{\partial x}{\partial \xi} \end{array}$$

THIS CAN BE  
OBTAINED

## ELEMENT STIFFNESS AND LOAD CALCULATIONS

$$K_{ij}^{(k)} = \int_{\hat{A}} \left( \left( \frac{\partial \hat{N}_j}{\partial \xi} \frac{\partial \xi}{\partial x} + \frac{\partial \hat{N}_j}{\partial \eta} \frac{\partial \eta}{\partial x} \right) \left( \frac{\partial \hat{N}_i}{\partial \xi} \frac{\partial \xi}{\partial x} + \frac{\partial \hat{N}_i}{\partial \eta} \frac{\partial \eta}{\partial x} \right) + \left( \frac{\partial \hat{N}_j}{\partial \xi} \frac{\partial \xi}{\partial y} + \frac{\partial \hat{N}_j}{\partial \eta} \frac{\partial \eta}{\partial y} \right) \left( \frac{\partial \hat{N}_i}{\partial \xi} \frac{\partial \xi}{\partial y} + \frac{\partial \hat{N}_i}{\partial \eta} \frac{\partial \eta}{\partial y} \right) \right) |J| d\hat{A}$$

$$F_i^{(k)} = \int_{\hat{A}} \left( \hat{r}(x(\xi, \eta), y(\xi, \eta)) \hat{N}_i \right) |J| d\hat{A}$$



# THE PLANAR ELASTICITY PROBLEM

## THE GLOBAL BALANCE LAW

$$\begin{array}{l} \boxed{r_x} \implies \sigma_{xx,x} + \sigma_{yx,y} + f_x = 0 \\ \boxed{r_y} \implies \sigma_{xy,x} + \sigma_{yy,y} + f_y = 0 \end{array} \left. \vphantom{\begin{array}{l} \sigma_{xx,x} \\ \sigma_{yy,y} \end{array}} \right\} \text{in } B$$

RESIDUE VECTOR  $\{r\}$   $\implies \int_B \{r\} \cdot \{v\} dA = 0$

WEIGHTED RESIDUAL FORM

WEIGHT FUNCTION OR VIRTUAL  
DISPLACEMENT VECTOR

$$\int_B \left[ \left( \sigma_{xx,x} + \sigma_{yx,y} + f_x \right) v_x + \left( \sigma_{xy,x} + \sigma_{yy,y} + f_y \right) v_y \right] dA = 0$$

↓ Integrate by parts

$$\int_B \left[ \left( -\sigma_{xx} v_{x,x} - \sigma_{yx} v_{x,y} + f_x v_x \right) + \left( -\sigma_{xy} v_{y,x} - \sigma_{yy} v_{y,y} + f_y v_y \right) \right] dA$$

$$+ \int_{\partial B} \left[ \left( \sigma_{xx} n_x + \sigma_{yx} n_y \right) v_x + \left( \sigma_{xy} n_x + \sigma_{yy} n_y \right) v_y \right] = 0$$

$T_x$

$T_y$

Boundary Traction

**WEAK OR VIRTUAL WORK FORMULATION**

$$\int_B \left[ \left( \sigma_{xx} \varepsilon_{xx}(v) + \sigma_{xy} \gamma_{xy}(v) + \sigma_{yy} \varepsilon_{yy}(v) \right) \right] dA =$$

$$\int_B \left( f_x v_x + f_y v_y \right) dA + \int_{\partial B} \left( T_x v_x + T_y v_y \right) dS$$

# GENERALIZED LINEAR ELASTICITY

## CONSTITUTIVE RELATION

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{16} \\ C_{12} & C_{22} & C_{26} \\ C_{16} & C_{26} & C_{66} \end{bmatrix} \begin{Bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{xy} \end{Bmatrix}$$

$\{\sigma(u)\}$

$[C]$

## STRAIN-DISPLACEMENT RELATION

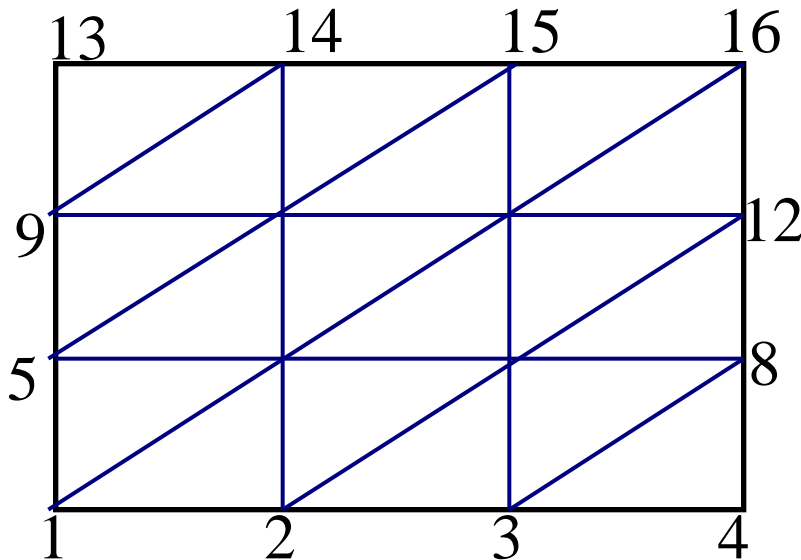
$$\begin{Bmatrix} \varepsilon_{xx}(u) \\ \varepsilon_{yy}(u) \\ \gamma_{xy}(u) \end{Bmatrix} = \begin{Bmatrix} \frac{\partial u_x}{\partial x} \\ \frac{\partial u_y}{\partial y} \\ \frac{\partial u_x}{\partial y} + \frac{\partial u_y}{\partial x} \end{Bmatrix}$$

$\{\varepsilon(u)\}$

## THE LEFT HAND SIDE OF THE WEAK FORM

$$\int_B \{\varepsilon(v)\}^T \{\sigma(u)\} dA = \int_B \{\varepsilon(v)\}^T [C] \{\varepsilon(u)\} dA$$

NEED  $C^0$  BASIS FUNCTIONS FOR BOTH  $u_x, u_y$



$$u_{x_{FE}} = \sum_{i=1}^{16} \alpha_i \phi_i(x, y)$$

$$u_{y_{FE}} = \sum_{i=1}^{16} \beta_i \phi_i(x, y)$$

THIS IS EQUIVALENT TO

$$\begin{Bmatrix} u_{x_{FE}} \\ u_{y_{FE}} \end{Bmatrix} = \begin{bmatrix} \phi_1 & 0 & \phi_2 & 0 & \phi_3 & 0 & \dots & \phi_{16} & 0 \\ 0 & \phi_1 & 0 & \phi_2 & 0 & \phi_3 & \dots & 0 & \phi_{16} \end{bmatrix} \begin{Bmatrix} \alpha_1 \\ \beta_1 \\ \alpha_2 \\ \beta_2 \\ \alpha_3 \\ \beta_3 \\ \vdots \\ \alpha_{16} \\ \beta_{16} \end{Bmatrix}$$



$$\{u_{FE}\} = [\phi]\{\alpha\}$$

$$\{v_{FE}\} = [\phi]\{\delta\}$$



The virtual displacement

$$\{\boldsymbol{\varepsilon}(u_{FE})\} = \left\{ \begin{array}{l} \mathbf{u}_{x_{FE},x} \\ \mathbf{u}_{y_{FE},y} \\ \mathbf{u}_{x_{FE},y} + \mathbf{u}_{y_{FE},x} \end{array} \right\} = [\mathbf{B}]\{\boldsymbol{\alpha}\}$$

$$[\mathbf{B}] = \begin{bmatrix} \phi_{1,x} & 0 & \phi_{2,x} & 0 & \phi_{3,x} & 0 & \dots \\ 0 & \phi_{1,y} & 0 & \phi_{2,y} & 0 & \phi_{3,y} & \dots \\ \phi_{1,y} & \phi_{1,x} & \phi_{2,y} & \phi_{2,x} & \phi_{3,y} & \phi_{3,x} & \dots \end{bmatrix}$$

$$\{\boldsymbol{\varepsilon}(v_{FE})\} = [\mathbf{B}]\{\boldsymbol{\delta}\}$$

$$\{\delta\}^T \left[ \int_B [B]^T [C] [B] \{ \alpha \} dA - \int_B [\phi]^T \{ f \} dA - \int_{\partial B} [\phi]^T \{ T \} dS \right] = 0$$



$$\left[ \int_B [B]^T [C] [B] dA \right] \{ \alpha \} = \int_B [\phi]^T \{ f \} dA + \int_{\partial B} [\phi]^T \{ T \} dS$$



$[K]$



2\*NNDOF X 2\*NNDOF sized  
stiffness matrix

NNDOF = 16 for the  
example